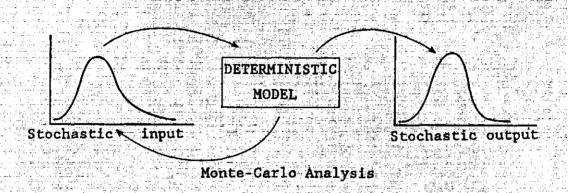
SOIL HYDRAULIC PROPERTIES IN THE STUDY AREA HUPSELSE BEEK AS OBTAINED FROM THREE DIFFERENT SCALES OF OBSERVATION: AN OVERVIEW



PUBLICATION 78

HYDRAULICA EN
AFVOERHYDROLOGIE
Landbouwuniversiteit

Wageningen

202521

SOIL HYDRAULIC PROPERTIES IN THE STUDY-AREA HUPSELSE BEEK AS OBTAINED FROM THREE DIFFERENT SCALES OF OBSERVATION: AN OVERVIEW

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Department of Hydraulics and Catchment Hydrology Agricultural University Wageningen, 1987

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Introduction

Soil properties vary in space. Especially when the area of interest classifies into various soil map units. However, also soils that are seemingly uniform or soils within a soil map unit can vary such that no representative value for a soil property can be found from one or a few samples (Warrick and Nielsen, 1980).

Water balance models and saturated/unsaturated water flow models most often require knowledge of the soil hydraulic properties of the system considered. In the past, one would determine one or a few representative water characteristic curves and hydraulic conductivity functions and use those for the model calculations. More intensive sampling has shown that soil properties can vary much more than we anticipated them to vary. The fact that in many cases water flow models are very sensitive to variation in soil hydraulic properties, therefore, poses a problem. An intensive measurement campaign has been set up in the study-area Hupselse Beek with 2 of the following objectives: (i) to determine the variation of the soil hydraulic properties in the study-area and (ii) to find techniques to describe this variation, so that it can be used as stochastic input in a numerical model to simulate soil-water flow.

The data, pertaining to all the measurements of soil hydraulic properties in the study-area form the basis of this report. Results of statistical analysis of the available soil hydraulic data will be presented in the second part.

Description of Measurement Sites

Soil hydraulic properties were determined for various horizons at three different scales of observation. In the first sampling scheme, seven profiles across the 650 ha study area were examined. These seven sites were chosen in such a way that they included most characteristic profiles and horizons that were classified in the study area. This classification was based on a densily spaced soil survey (1200 points). The seven sites will be referred as sampling scheme 1. Figure 1 shows a 1:25000 map of the Hupselse Beek area with the location of the seven measurement sites. The names of the sites and their respective sampled horizons are listed in Table 1. The results of the soil physical measurements were reported by Wösten et al. (1983).

For the second sampling scheme, an area of 0.5 ha was chosen such that the seven sites within this area were all from the same and most important soil map unit (Hn52-STIBOKA soil map). Each of the seven sites was sampled in duplo at the 10 and 50 cm soil depth. A detailed map of the sub-area with the sampled locations is shown in Figure 2. To the northeast within this sub-area lies site 1 of sampling scheme 1. Brom (1983) reported the soil physical data for this sampling scheme.

Finally, the highest sampling density was achieved in the third sampling scheme, where at six locations triplicate samples were taken within 2 m². This sampling area was located between sites 1 and 7 of sampling scheme 2 (Fig. 2). At each of the six locations, samples were taken at depths of 30, 60 and 90 cm. Figure 2 shows the location of the sampling area, while a schematic view of the sampling strategy of sampling scheme 3 is presented in Figure 3. Booltink (1985) gives a detailed presentation of the measurement techniques and the soil physical data pertaining to the last sampling scheme.

		Sampled horizons					
Site	Name	Soil Water Characteristic Curve	Hydraulic Conductivity Function				
1	Brom pv	A1 B2 C11 C12	B2 C11 C12				
2	Assink pv	A1 B2 C11 C12	B2 C11 C12				
3	Lensink	Aan B2 C11 C12	B2 C11 C12				
4	Assink bl	A1 B2 C11 C12	B2 C11 C12				
5	Ten Barge	A1 B2 C11g D1 D2	B2 C11g D1 D2				
6	Schuurmans	A1 D1	D1				
7	Faaks	Ap C11g D1	C11g D1				

Table 1. Measurement sites and sampled horizons for sampling scheme 1.

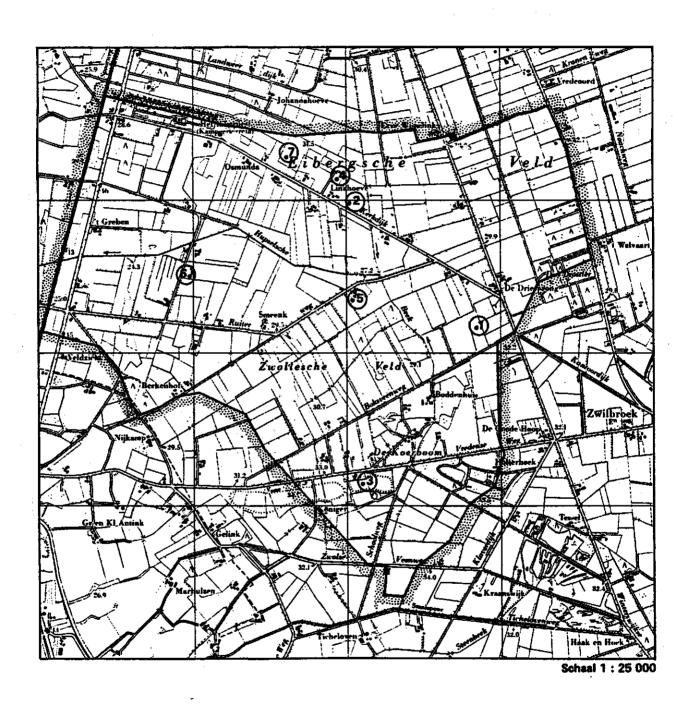


Figure 1. Sample locations of sampling scheme 1.

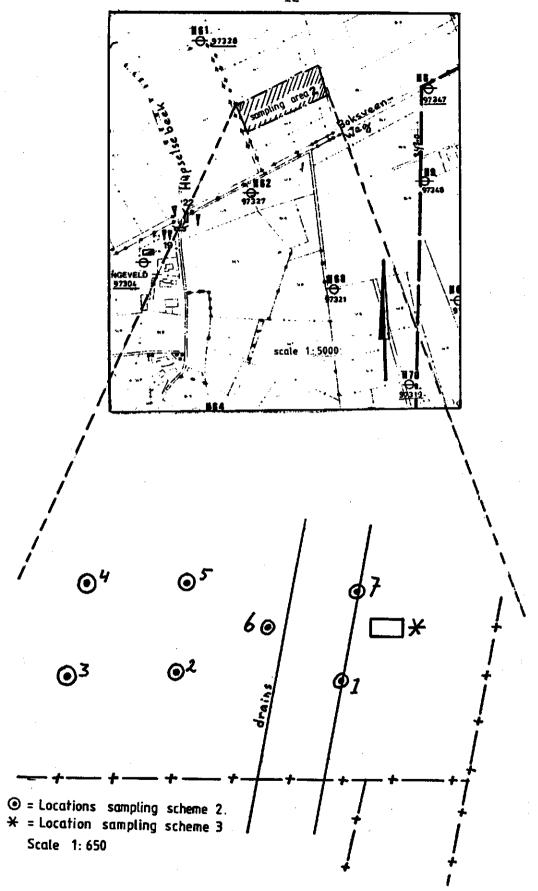


Figure 2. Location of sampling schemes 2 and 3 and the measurement sites of scheme 2.

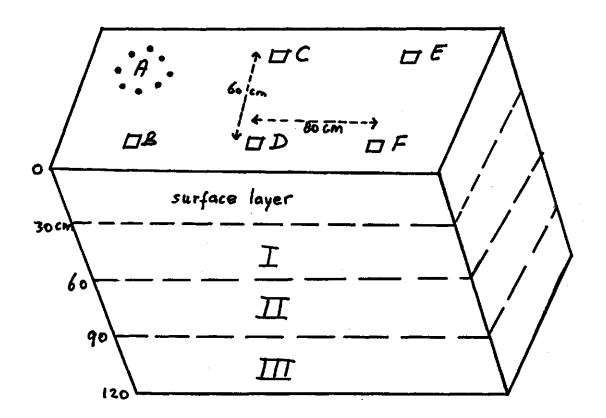


Figure 3. Sampled locations of sampling scheme 3.

Materials and Methods

Soil water characteristic curves as well as hydraulic conductivity functions were obtained by various techniques.

Stiboka (Wösten et al., 1983; sampling scheme 1) determined soil water characteristic curves by slow evaporation of wet undisturbed samples in the laboratory, in combination with in-situ measurements. In both cases, soil water pressure heads (h) were determined from tensiometer readings, while water contents (θ) were obtained from gravimetric sampling and neutron probe readings, respectively. Hydraulic conductivity (K) values of soil above the water table were measured with the crust-test down to approximately h--50cm (Bouma et al., 1977) and by the sorptivity method (Dirksen, 1979) and the hot-air method (Arya et al., 1975) for lower K-values. The samples contained in PVC-cylinders to be used for the crust-test were also used for the laboratory part of the soil water characteristic curves. Wösten et al (1983) reported very good agreement between the hydraulic conductivity values calculated with the sorptivity and hot-air method.

Brom (1983; sampling scheme 2) used the sandbox apparatus (Stakman et al., 1969) to determine soil water characteristic curves down to a soil water pressure head of approximately -500 cm. Undisturbed soil cores were thereby put on a box filled with sand or kaolien clay, while the desired suction was applied to the sandbox by a hanging water column or a suction pump. A continuous water phase will be established, unless the air-entry value of the soil in the sandbox is smaller than the desired suction in the soil sample. Hydraulic conductivity values as a function of soil water pressure head were again measured by the crust-test, while lower K-values as a function of 0 were determined by both the sorptivity and hot-air method. However, results obtained by the sorptivity method deviated substantially from the K-data as determined with the hot-air method. In addition, there was a poor match between the K-values determined with the crust method at high water content values and the data obtained from the sorptivity method in the lower content range.

Booltink (1985; sampling scheme 3) used the same techniques as Brom (1983). However, after desorption of the soil sample, Booltink also determined the sorption part of the soil water characteristics.

Analysis of so many soil physical data would be easier if the data can be fitted by analytical expressions. Van Genuchten (1978) introduced closed-form analytical expressions for both hydraulic functions. These are:

$$\Theta = \left[\frac{1}{1 + |\alpha h|^{n}}\right]^{m} , \qquad [1]$$

and

$$K_{rel}(\theta) - K/K_s - \Theta^{1/2} \left[1 - (1 - \Theta^{1/m})^m\right]^2$$
, [2]

where

$$\Theta = \frac{\theta - \theta_{r}}{\theta_{s} - \theta_{r}}, \text{ and } m = 1 - (1/n).$$

 $\theta_{\rm r}$ refers to the residual water content for which the slope ${\rm d}\theta/{\rm d}h$ becomes zero, excluding the region near $\theta_{\rm S}$, the saturated water content. ${\rm K_S}$ denotes the saturated hydraulic conductivity. Equations [1] and [2] therefore constitute a 5-parameter model (α , n, $\theta_{\rm S}$, $\theta_{\rm r}$ and ${\rm K_S}$).

Van Genuchten (1978) developed a curve fitting procedure to estimate the parameters $\theta_{\rm T}$, α and n from available water retention data. It is thus assumed that $\theta_{\rm S}$ is known. These parameters together with a measured $K_{\rm S}$ -value can then be used in Equation [2] to describe the hydraulic conductivity function. The described Van Genuchten fitting procedure was used, except that $\theta_{\rm T}$ was not estimated but set equal to zero in all cases. No satisfactory $\theta_{\rm T}$ -value could be estimated as in almost all cases no water retention data were available for the dryer part of the soil water characteristic curves (beyond the inflection point of the curves). Since $\theta_{\rm T}$ is known, van Genuchten's curve fitting program was modified such that instead of $\theta_{\rm T}$, an optimum value for $\theta_{\rm S}$ could be estimated ($\theta_{\rm S}^*$).

However, rather than using a measured $K_{\rm S}\text{-value}$ in Eq. [2], a conductivity value at some intermediate water content was used to obtain an optimum $K_{\rm S}^{\star}\text{-}$

value such that Eq. [2] would match the experimental data points best. In an iterative way successive $K(\theta)$ -combinations were fitted to Van Genuchten's model. The combination that yielded the minimum least squares was assumed to be the optimum hydraulic conductivity function. To reduce the weight of the points at high K-values, the logarithm of K instead of K was used in the optimalization procedure.

Description Datafile

The soil hydraulic data pertaining to all three sampling schemes were combined in one datafile. Each sampled site was given a soil identification number, consisting of four digits. The fourth digit indicates whether the soil physical data for that particular site and horizon are combined (1), or that each replicate is considered separately (digit refers to sample number). Table 2 explains the meaning of each digit. The structure of the data file is shown in Table 3, which lists the data for soil number 3331. The soil identification number is followed by the x, y and z coordinates of the site. X and y are given in meters, while the z-coordinate is expressed in cm depth below the soil surface.

The first two numbers on the next data line refer to the available number of experimentally determined water retention and hydraulic conductivity data. These are followed by a value for α , n, θ_s^* and K_s^* , to be used when one prefers Van Genuchten's analytical expressions (Equation [1] and [2]). The superscript star refers to fitted, rather than measured θ_s and K_s -values. The fitting procedure assumes θ_r to be zero. A K_s^* -value of 99.9999 denotes a missing value. It also indicates that no unsaturated conductivity data are available.

Finally the last two values on the second data line denote α and n for the sorption part of the soil water characteristic. Sorption data were only determined in sampling scheme 3.

The following set of lines contain the experimentally determined $(\theta - h)$ -combinations $(\theta \text{ in cm}^3 \text{ cm}^{-3} \text{ and h in cm})$. The number of data points is defined in the second data line. All the remaining lines refer to hydraulic conductivity data. In general, these data can be divided into three groups. K-data obtained with the crust-test, the sorptivity method and the hot-air method. Each line lists the hydraulic conductivity (cm day⁻¹), the corresponding h (cm) and θ -value, and for K-data determined with the last two methods also the diffusivity value (cm² s⁻¹). Only for sampling scheme 3, the K-data obtained by the sorptivity method were calculated using the

sorption part of the water retention curve. No sorption data were collected for the other 2 schemes.

For sampling schemes 2 and 3, the data file contains also the soil water retention data for the individual samples (3332 and 3338 in Table 3). Since each sample was either used for water retention or conductivity measurements, no K-data are here included. That is, water retention and conductivity data were never determined from the same sample.

The data file can be read by the format descriptions listed in Table 4. The data file is included in the Appendix.

Digit	Description	Possible values
1	sampling scheme	1, 2, 3
2	site number	1, 2, 3, 4, 5, 6, 7
3	depth indication	1, 2, 3, 4, 5
4	sample number	1*, 2, 3, 5, 8

Table 2. Description of soil identification number.

3331	242508.6	453081.9	90.0				1 (070
13	30		2.1261	0.2930	53.5	0.0621	1.6279
0.278	3						
0.262	32						
0.202	63						
0.112	100						
0.066	148						
0.036	331						
0.311	3						
0.292	10						
0.279	32						
0.230	63						
0.172	100					•	
0.144	148						
0.081	331						
272.2200	0			CRUST			
192.8700	10	0.290					
175.3400	14	0.287					
51.6000	24	0.277					
22.4500	18	0.284					
122.4000	19	0.283					
104.3600	2	0.293					
250.6300	0	0.293					
0.0170	204	0.059	0.00110	SORP			
2.5332	72	0.110	0.03300				
19.6759	41	0.150	0.12000				
8.8477	54	0.130	0.07700				
45.0111	28	0.180	0.18000				
0.0007	258	0.051	0.00007	•			
0.0016	210	0.058	0.00011				
0.1272	100	0.091	0.00270	*			
0.5415	85	0.100	0.00900				
62.0486	54	0.130	0.54000				
0.0014	340	0.043	0.00021				
0.0312	162	0.068	0.00140				
1.0747	72	0.110	0.01400				
27.7928	25	0.190	0.10000				
0,9719	73	0.190	0.00690	HAM			
0.9400	87	0.170	0.00770				
0.6579	102	0.150	0.00640				
0.3040	121	0.130	0.00370				
0.0543	145	0.110	0.00087				
0.0188	179	0.090	0.00043				
0.0070	228	0.070	0.00025				
0.0022	313	0.050	0.00015				
2220	242508.8	453082.0	90.0				
3332		0.0146	2.7256	0.2806	53.5		
6 0.278	0 3	0.0146	2.7236	0.2800	ر.در		
0.262	32						
0.202	63						
0.112 0.066	100 148						
0.036	331 242508.6	453081.7	90.0				
3338 7	242308.6	0.0157	1.8078	0.3062	53.5		
0.311	3	0.0137	1.00/0	0,3062			
0.311	10		•		•		
+ +	32						
0.279	63						
0.23	100						
0.172 0.144	148						
0.144	331						
0.001	JJ1						

Table 3. Soil physical data for site 333.

Line/data	Format
1	5x, A5, 3(F10.1)
2	2110, 6F10.4
retention	F10.3, I10
conductivity	F10.4, I10, F10.3, F10.5

Table 4. Format description data file.

Discussion

Diffusivity values D, obtained by the sorptivity and hot-air method, are used to compute hydraulic conductivity values. The two properties are related by:

$$K(\theta) = D(\theta) * C(\theta),$$
 [3]

where $C(\theta)$ denotes the water capacity function or the slope of the water characteristic curve. Using the same characteristic curve one can subsequently determine K(h)-combinations. Therefore, only those conductivity data are presented that correspond to soil water pressure head values equal or larger than for which soil water characteristic curves were determined.

Since the soil water characteristic function is hysteretic, so is the diffusivity function $D(\theta)$. There is, therefore, a marked difference between the sorptivity and hot-air method. In the first method, water is absorbed by the soil, while in the latter hot-air method the soil is dried. In principle, $K(\theta)$ and K(h)-data obtained by the two methods can only be combined if both the wetting and drying part of the soil water characteristic are measured $(C_W(\theta))$ and $C_d(\theta)$, where subscripts w and d denote wetting and drying, respectively). No sorption data, however, were measured by either Wösten et al. (1983, sampling scheme 1) or Brom (1983, sampling scheme 2). Wösten et al. (1983) found still good agreement between the two methods, when K was plotted versus h. However, $K(\theta)$ and K(h) data obtained with the sorptivity method by Brom (1983) did in most cases not agree with those determined with the hot-air and crust-method. These K-data were therefore eliminated and are not presented in this report.

Booltink (1985, sampling scheme 3) did measure hysteresis. Examples for two soils are shown in Figure 4 and 5. The same figures also show K(h)-data as obtained with the sorptivity method when the desorption (circles) or sorption part (+) of the soil water characteristic curve is used to convert from diffusivity to conductivity values (Equation [3]). These figures show

that there is a significant hysteresis effect, which should be considered when soils are either wetted or dried to determine conductivity values. It can be seen that the sorptivity method will tend to overestimate K when using the desorption or drying curve. Booltink's diffusivity data to calculate K were indeed treated as being partly obtained during drying (hot-air method) and partly during wetting (sorptivity method).

The resulting $K(\theta)$ and K(h) relationships for the same samples are shown in Figure 6 and 7. The $K(\theta)$ -data are divided into three groups, each group being determined by another method. The data obtained with the crust-test (circles) and hot-air method (+) seem to overlap well.

There is, however, no agreement with the sorptivity method (triangles) at higher water content values. A similar inconsistancy between the sorptivity method and the other two methods was reported by Brom (1983). The sorption data indicate a 1000-fold increase in K with a water content increase of ca. 0.05 (Figure 6b and 7b) in the high conductivity range. This seems very unlikely. A continuous slip on the cam may have resulted in a decreased infiltration rate and therefore in too low water content values. Therefore, the $K(\theta)$ -data from the sorptivity method were not included in the fitting of Van Genuchten's expression (Equation [2]).

Since the K(h)-function is hysteresis dependent, there exists no unique relation between K and h. The fitted curves (van Genuchten) through the K(h)-data in Figure 6c and 7c are desorption curves. The sorption curves would lie below the fitted lines.

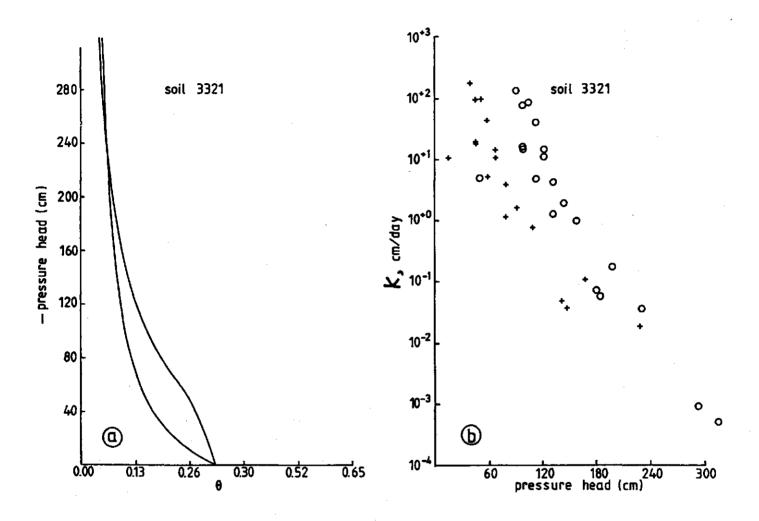


Figure 4. Drying and wetting curves (a) and K(h)-data from sorptivity method (b), when drying (circles) or wetting curve (+) is used for soil 3321.

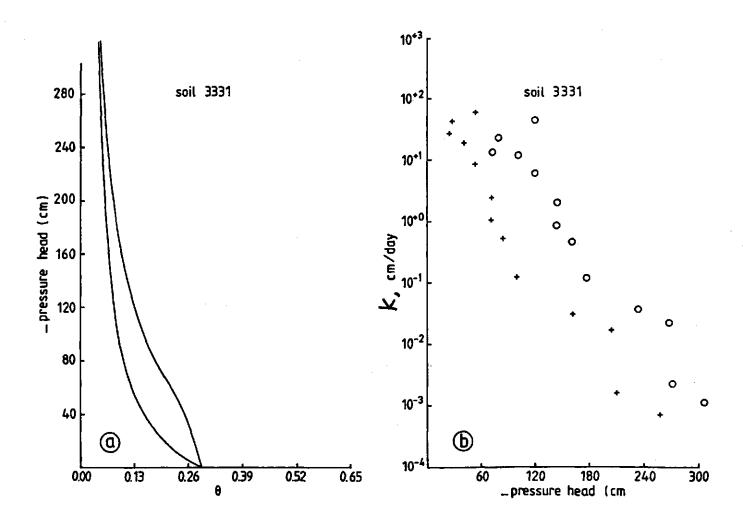
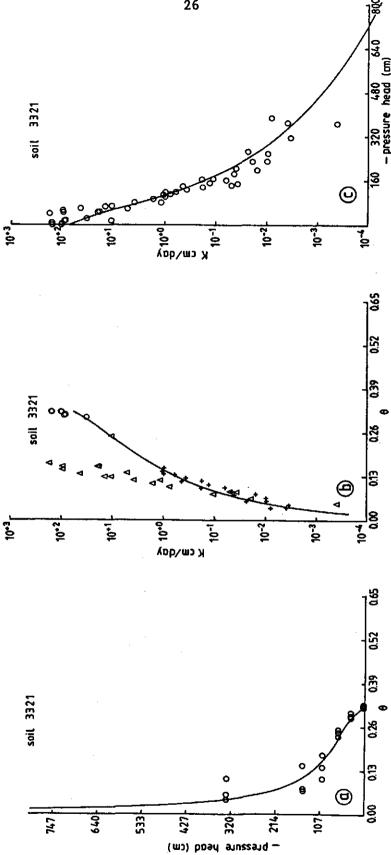


Figure 5. Drying and wetting curves (a) and K(h)-data from sorptivity method (b), when drying (circles) or wetting curve (+) is used for soil 3331.





Drying curve (a), $K(\theta)$ -data (b) as determined with crust test Figure 6. (circles), sorptivity method (triangles) and hot-air method (+) and combined K(h)-data (c), where lines are only fitted through data obtained with crust and hot-air method. Soil 3321.

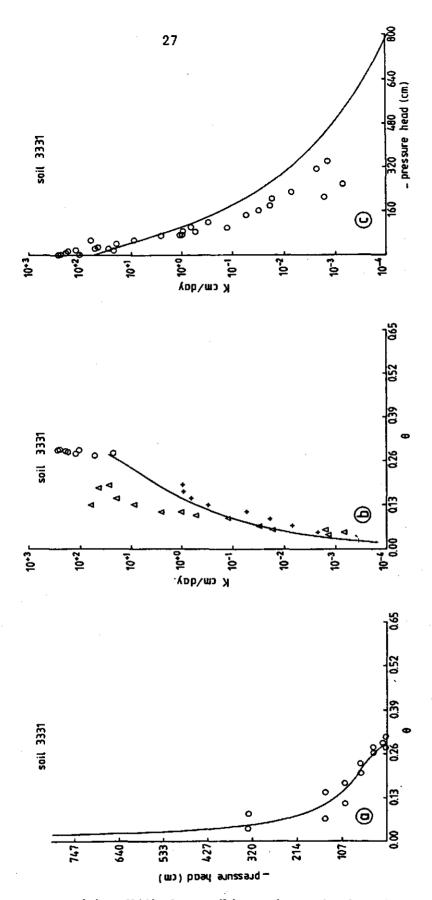


Figure 7. Drying curve (a), $K(\theta)$ -data (b) as determined with crust test (circles), sorptivity method (triangles) and hot-air method (+) and combined K(h)-data (c), where lines are only fitted through data obtained with crust and hot-air method. Soil 3331.

Some Statistical Analysis

The following section gives some preliminary results from statistical analysis of the available soil physical data. The analysis is by no means complete, but is an indication of what can be done.

1. Multiple regression analysis

Multiple regression analysis was carried out to check if there exists significant correlation amoung α , n (Van Genuchten functions), K_s^* and θ_s^* . High correlation coefficients were obtained when α was regressed against n, n^2 , K_s^* and K_s^{*2} . The results are listed in Table 5. The coefficients of determination (R^2) for sampling scheme 2, 3 and the combination of the two schemes are significantly larger than the other regressed populations. The first sampling scheme pertains to different soil map units, while the other two schemes comprised only one soil map unit.

A similar regression analysis was done for K_S^* being the dependent variable, while including θ_S^* as one of the independent variables. This is more interesting than the previous analysis, since prediction instead of measurement of K_S would reduce the total number of measurements required to quantify the soil physical characteristics. The results are shown in Table 6 and 7. Table 6 lists R^2 values for a stepwise increase in the number of independent variables, while Table 7 lists the regression coefficients when all independent variables are included. Table 6 clearly shows a significant increase of the R^2 values when θ_S^* is included in the regression analysis. The increase in the coefficient of determination with the inclusion of $(\theta_S^*)^2$ may be the result of the quadratic relation between saturated hydraulic conductivity and pore size radius. Figure 8 illustrates how such predicted K_S -values compare with the fitted K_S -values for sampling scheme 1.

Sampl:	ing sch	eme	Regression	n Coefficien	ts		R ² -Value
		a	b	c	d*10 ³	e*10 ⁶	
1	(19)¶	.0978	-0.588	.00825	.150	-0.100	.749
2	(14)	00301	.00690	00036	.291	-0.300	.897
3	(17)	.0696	0522	.0114	.162	-0.300	.863
1+2	(33)	.0511	0291	.00422	.132	-0.100	.639
2+3	(31)	.0301	0221	.00566	.199	-0.200	.819
1+3	(36)	.0662	0372	.00528	.135	-0.100	.709
1+2+3	(50)	.0422	0214	.00307	.125	-0.000	.643

Table 5. Regression coefficients and coefficient of determination for prediction of α from n and K_s^* -values $(\alpha = a + b \cdot n + c \cdot n^2 + d \cdot K_s^* + e \cdot K_s^{*2}).$

independent		R	2-Values	For Samp	ling Sche	me	
parameters	1	2	3	1+2	1+3	2+3	1+2+3
α, n	. 464	. 520	.802	.483	.495	.579	. 500
α , n, θ_s^*	.819	.665	.803	.701	.724	.596	,658
α , n, α^2 , n ² α , n, θ_s^* ,	.773	.557	.827	.652	.711	. 586	.631
α^2 , n^2 , θ_s^{*2}	.974	.792	.878	. 870	.929	. 624	.847

Table 6. Coefficients of determination for prediction of K_S^* from α , n and θ_S^* .

			Regres	sion coef	ficients			R²
Sampling scheme	a*10 ⁻³	b*10 ⁻⁴	c*10 ⁻¹	d*10 ⁻⁴	e*10 ⁻⁴	f	g*10 ⁻⁴	
1	.2586	2.503	40.56	8344	-10.40	-49.43	1.297	.974
2	8.715	.1089	-1299	1.577	4.011	3823	-2.732	.792
3	-1.522	-1.798	-18.49	1.084	42.39	43.51	-1.467	.878
1+2	2.031	1.714	-7.653	-1.326	-7.587	-10.42	1.833	.870
1+3	1.337	2.097	28.19	-1.204	-8.905	-36.15	1.699	.929
2+3	.1459	. 9896	-53.93	. 2575	-3.481	111.0	4536	.624
1+2+3	2.019	1.583	1.348	-1.279	-6.796	5399	1.774	. 847

Table 7. Regression coefficients and coefficients of determination for prediction of K_S^* from α , n and θ_S^* $(K_S^* = a + b \cdot \alpha + c \cdot n + d \cdot \theta_S^* + e \cdot \alpha^2 + f \cdot n^2 + g \cdot \theta_S^{*2})$

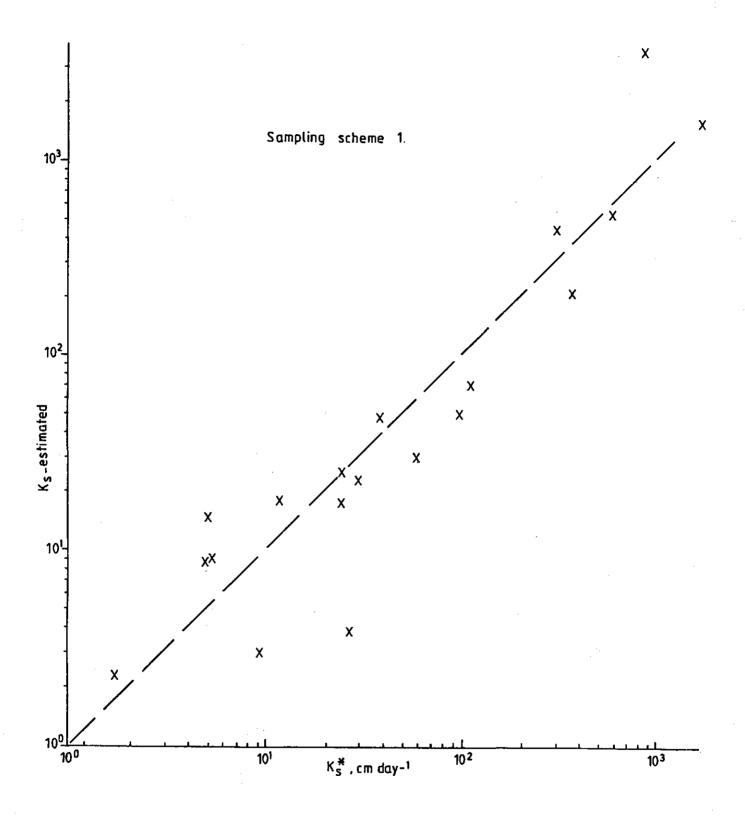


Figure 8. Saturated hydraulic conductivity values of sampling scheme 1 predicted from α , n, and θ_s , plotted versus K_s^* .

2. Test for distribution type

The study of soil water flow with spatial variable soil hydraulic properties requires that the distributions of the values of the properties or of the parameters that functionally describe these properties are known. This is true since the soil hydraulic properties will then serve as stochastic input for a computer model to simulate unsaturated water flow. It is, therefore, of interest to find the distribution function of α , n, θ_S^* and K_S^* . In Table 8 a normal and lognormal distribution are compared for these four variables and for the various sampling schemes. In this table, KS denotes the modified distribution-free Kolmogorov-Smirnov Statistic (Stephens, 1974), which is to determine the goodness-of-fit of a hypothetized theoretical distribution with an estimated mean and variance to the empirical distribution function. The KS-statistic is a quantitive measure of the maximum difference between the empirical and hypothetical distribution function, and its value therefore decreases if the 2 distributions are closer together. In the case treated in Table 8, a value below .895 indicates an acceptable fit at the 5% probability level (see Stephens, 1974).

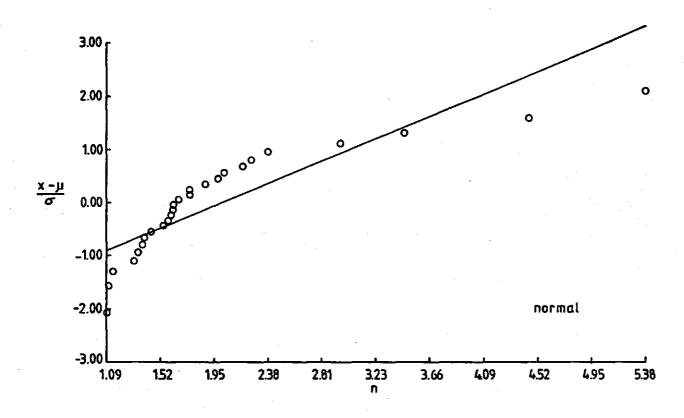
According to Stephens (1974) KS was calculated from $D^*[n^{0.5}-0.01+0.85/n^{0.5}]$, where D^* is the usual Kolmogorov-Smirnov statistic and n the number of observations. Those parameters that are labelled with a star were rejected as being normally or lognormally distributed at the 95% confidence level. In general, K_S^* follows a lognormal and θ_S^* a normal distribution function. One can also observe from the last two columns in Table 8 that the KS-statistic largely decreased in most cases when the data were transformed to a lognormal distribution. Visual inspection of the frequency distributions, (Fig. 9 to 12) would indicate that a lognormal distribution fits the empirical data better for all parameters, except possibly θ_S^* . A similar conclusion was reported by Greminger et al. (1985). Examples of such distributions are shown in Figure 9 through 12 for sampling scheme 1.

	μ		σ		KS	
parameter	normal	lognorm.	normal	lognorm.	normal	lognorm.
Sampling scheme 1						
α (26)	.03440	-3.686	.03405	.8023	1.382*	.652
n (26)	2.011	.6093	1.023	.4008	1.183*	.867
θ_{s}^{\star} (26)	. 3778	9899	.07322	.1829	.741	.594
K _s (19)	236.2	3.614	455.5	2.502	1.554*	.709
Sampling scheme 2						
α (14)	.02264	-4.015	.01756	.6729	.859	.546
n (14)	1.440+	3593+	.1615	.1057	.970*	.898
θ_{s}^{\star} (14)	. 3689	-1.005	.04731	.1327	.451	.538
K_{s}^{*} (14)	116.3	3.709	198.3	1.485	1.291*	.408
Sampling scheme 3						
α (17)	.02382	-3.822	.01076	.4119	1.136*	1.097*
n (17)	1.947	. 6423	.4606	. 2227	1.062*	.963*
θ_{s}^{\star} (17)	.3383+	-1.096+	.05631	.1589	1.097*	1.002*
K _s (17)	94.01	4.066	96.35	1.038	1.473*	.721
Sampling scheme 2 + 3						
α (31)	.0233	-3.909	.01399	. 5443	1.250*	.930*
n (31)	1.718	.5145	.4363	. 2276	1.165*	.874
θ_s^* (31)	.3522	-1.055	.05386	.1523	.748	.634
K_s^* (31)	104.1	3.905	148.7	1.250	1.742*	.531
Sampling scheme 1 + 2 + 3						
α (57)	.02835	-3.807	.02557	.6772	1.576*	.643
n (57)	1.852	.5578	.7685	.3189	1.768*	1.331*
θ_s^* (57)	. 3639	-1.025	.06414	.1686	.727	.627
K _s (50)	154.3	3.794	306.5	1.809	2.233*	. 546

mean is significantly different from sampling scheme 1 at 95% confidence level.

Table 8. Comparison of normal and lognormal distribution functions for α , n, θ_s^* and K_s^* (all sampling depths combined).

^{*} hypothesis that distribution is normal or lognormal is rejected at 95% confidence level (critical region: KS<0.895).



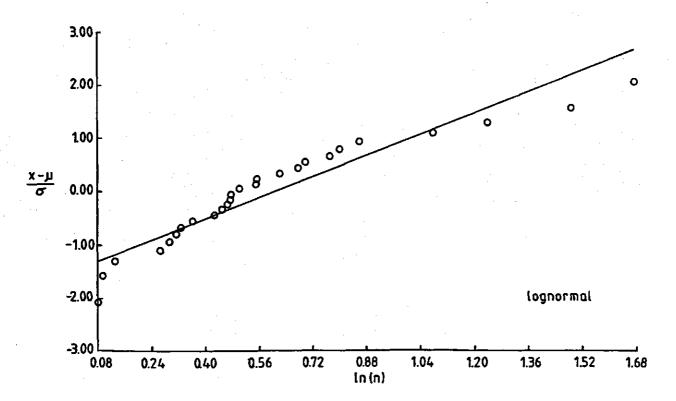
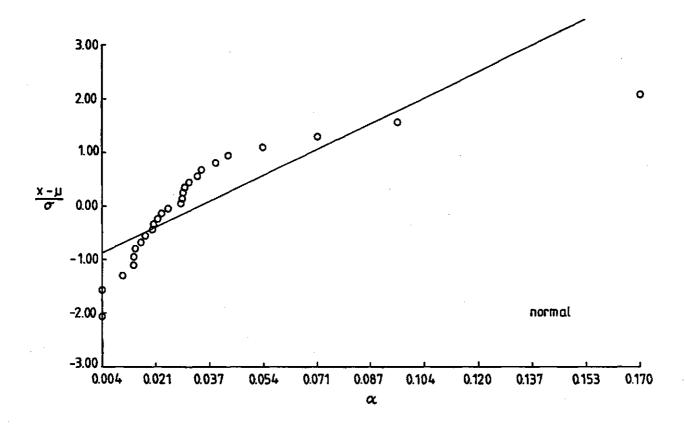


Figure 9. Fractile diagrams for normal and lognormal distribution of n (scheme 1).



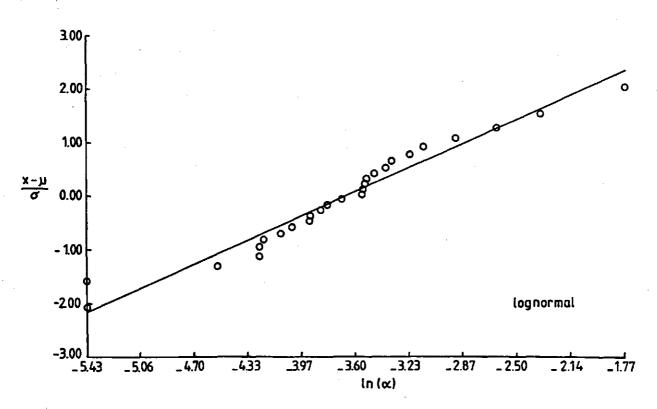


Figure 10. Fractile diagrams for normal and lognormal distribution of α (scheme 1).

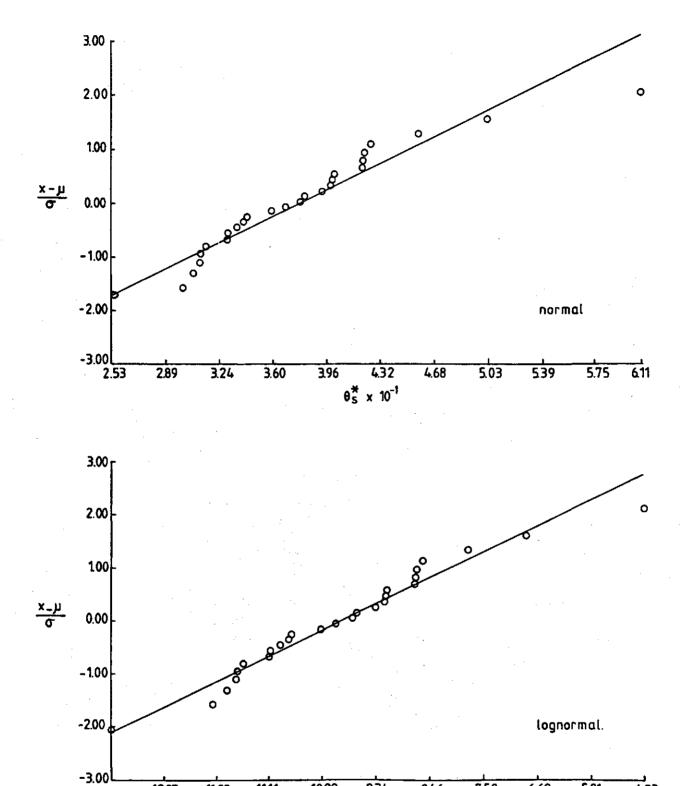


Figure 11. Fractile diagrams for normal and lognormal distribution of $\theta_{\rm S}$ (scheme 1).

-9.34 -846 ln (0*x) x 10⁻¹ -7.58

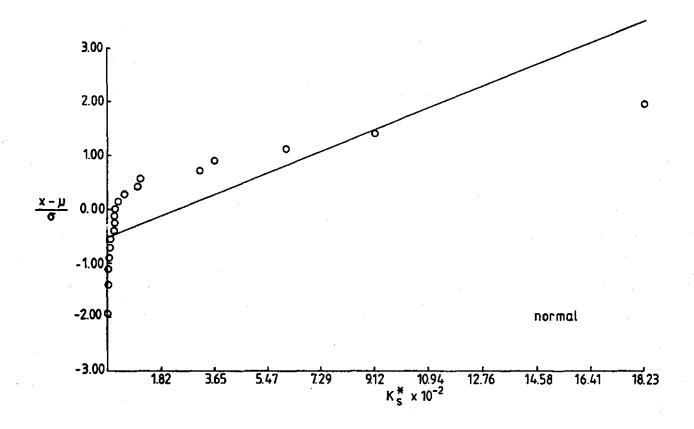
-5.81

-10.22

-11.99

-12.87

-11.11



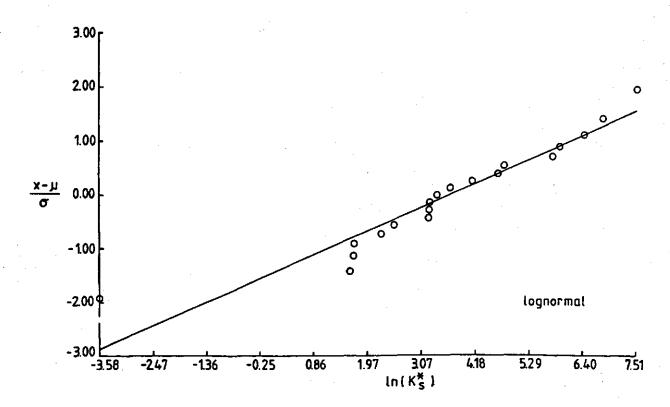


Figure 12. Fractile diagrams for normal and lognormal distribution of $K_{\rm S}$ (scheme 1).

Also the gamma distribution function was considered as a possible distribution function. A comparison of the normal, lognormal and gamma distribution is shown in Table 9, which lists the sum of squares of the difference between the empirical and hypothetical distribution function for all four parameters when all 3 schemes are combined. It is clear from this table that the lognormal distribution is the best possible choice of the three considered (minimum sum of squares).

It is further of interest to know whether the parameters of sampling schemes 1, 2 and 3 are populations of the same normal or lognormal distribution function. The T-test can be used to test for equality of means. It should be noted that the T-test assumes that the samples under consideration are distributed independent. approximately normal and Ιf log-transformed values of the parameters were used in the T-test. independence assumption may not be fulfilled for parameters of the third sampling scheme, since soil samples were taken within an area of only 2 m2. At the 95% confidence level, only the mean of log (n) of sampling scheme 2 and the mean θ_s^* of sampling scheme 3 were significantly different from the respective means of sampling scheme 1 (Table 8).

3. Analysis of variance

Equality of variances between sampling schemes can be tested with the F-test. Comparison of the F-statistic values indicated that there is a significant difference in variance for most parameters. Therefore, the parameters of sampling schemes 1, 2 and 3 have different distribution functions. The F-test also indicated that the variances of all parameters of sampling scheme 2 were significantly smaller than of sampling scheme 1.

	Sum of Squares-Values					
Distribution	α	n	θ's	K*		
Gamma	.4511	. 5452	.0669	. 9097		
Normal	.8367	.8199	.0917	1.628		
Lognormal	. 0645	.3081	.0699	.0634		

Table 9. Sum of squares for comparison of three distribution functions.

Since different soil map units were part of the first sampling schemes this comes as no surprise. However, no such clear difference was found in the variances of sampling schemes 2 and 3, where only the variance of $\log \alpha$ was significantly smaller for the latter sampling scheme. It can furthermore be seen from Table 8 that the variances of n and θ_S^* of sample scheme 3 are larger than of scheme 2. This seems contradictory, since the sampled area of scheme 2 is much larger than of sample scheme 3 (5000 and 2 m², respectively), while both schemes 2 and 3 were part of the same soil map unit.

Sofar, all sampled depths were combined in the analysis. It seems, however, likely that significant differences in soil hydraulic properties will be found between horizons. Further analysis will focus on saturated water content and hydraulic conductivity. Values of these two variables for the various horizons and sampling schemes are listed in Table 10 and 11. Replicates $\theta_{\rm S}^{\star}$ -values were available for scheme 2 and 3, and no experimental $K_{\rm S}$ -data were determined for the A-horizon of scheme 1. No D-horizons were included in the analysis, since these occurred only in 3 sites of scheme 1. The difference of number and type of horizons between scheme 1 and the schemes 2 and 3 is the reason that only the A and B horizon of sampling scheme 1 are listed in Table 10 and 11. For sampling scheme 1 only 4 observations were available from C1- and C2-horizon, which each came from different depths.

Table 10. Available Replicate values of θ_s^{\star}

<u>scheme</u>	<u>location</u>		horizon		
		A	В	C	
1	1	.4003 (10)*	.4207 (35)		
	2	.4200 (10)	.3120 (60)		
	3	.5026 (40)	.3820 (95)		
	4	.3992 (15)	.3410 (50)		
	5	.4011 (15)	.3363 (65)		
	6	.4214 (20)			
	7	.3788 (15)	.3073 (40)		
2	1	.4151 .3901 (10)	.2915 .2676 (50)		
	2	.3776 .3803 (10)	.3595 .3039 (50)		
	3	.4826 .4107 (10)	.3452 .3633 (50)		
	4	.3986 .3841 (10)	.3972 .4107 (50)		
	5	.3888 .3173 (10)	.3305 .3285 (50)		
	6	.4194 .3727 (10)	.2973 .2963 (50)		
	7	.4389 .4120 (10)	.3827 .3676 (50)		
3	1	.4536 (32)	.3973 .3010	.3156 .3469	
			.3623 (64)	.2898 (90)	
	2		.3081 .3195	.3142 .3198	
			(64)	(90)	
	3	.4312 .4222	.3242 .3153	.2806 .3062	
		(35)	.3312 (60)	(90)	
	4	.4075 .4304	.3529 .2735	.2957 .2989	
		(35)	.2907 (60)	.2939 (90)	
	5	.3700 .4400	.2999 .3319	.2687 .2751	
		.3930 (28)	.2862 (59)	.2767 (90)	
	6	.3700 .3639	.2998 .2819	.2880 .2538	
		.4090 (28)	(59)	.2847 (90)	
depth ((cm)				

Table 11. Available K -values.

scheme	location		horizon	
		A	В	С
1	1		25.04	
	2		25.3	
	3		911.0	
	4		24.8	
	5		60.7	
	6	•	• • •	
	7		318.0	
2	1	10.30	3.53	
	2	80.13	9.92	
	3	25.46	695.0	
	4	96.33	420.19	
	5	11.03	138.50	
	6	31.14	19.06	
	7	40.06	47.32	
3	1	280.5	323.4	31.6
	2		73.2	44.9
	3	72.0	70.9	53.5
	4	75.5	62.2	45.5
	5	212.3	18.8	8.1
	6	190.6	18.3	16.9

Since replicate values of θ_S^* were available, the first point of interest was to test whether the location means of θ_S^* for a given sampling scheme and horizon are identical. The method to be used is called analysis of variance (ANOVA). The resulting F-test will provide a means to test whether fixed or random effects of each location are present (Snedecor and Cochran, 1980). Also, ANOVA is based upon the assumptions concerning normality and independency. Values for F, P (critical level), and LSD (least significant difference at 5% critical level) are shown in Table 12. The difference between a specific pair of means is significant at the 5% level if it exceeds LSD (Snedecor and Cochran, 1980). In only 2 out of 5 cases (scheme 2, B-hor., and scheme 3, C-hor.) a significant difference between locations was found. In the other 3 cases the variation between locations was not significantly larger than the within location variation.

Analysis of variance was also used to test whether the mean values of θ_s^* were identical for the different horizons and sampling schemes. The test results are shown in Table 13. When each sampling scheme was treated separately, the mean values of θ_s^* were significantly different for each horizon (Table 13, part A). On the other hand, when each horizon was treated separately, the mean values of θ_s^* were identical for each sampling scheme (Table 13, part B). One can therefore treat the whole population of θ_s^* -values (Table 10) as 3 different sub-populations, one for each horizon. The mean θ_s^* -values for the A, B and C horizons are 0.406, 0.331, and 0.294, while the corresponding standard deviations are 0.0354, 0.0401 and 0.0227, respectively.

Table 12. F, P, and LSD values to test whether location means of θ_s^* are identical.

<u>Scheme</u>	<u>Statistics</u>		<u>Horizon</u>			
	A	В	C			
2	F	1.85	11.88*			
	P	.219	.0023			
	LSD	.075	.042			
3	F	1.99	1.29	3.66*		
	P	.216	.340	.0385		
	LSD	.074	.064	.036		

^{*} F is significant at 5% level.

Table 13. F, P, and LSD values to test whether horizon and sampling scheme means of $\theta_{\rm S}^{\star}$ are identical.

A. Test for identical horizon mean:

<u>Scheme</u>	F	P	LSD
1 (A,B)	12.00*	0.0134	0.048
2 (A,B)	22.60*	<0.0001	0.0265
3 (A,B,C)	85.46*	<0.0001	0.0186
1+2+3	64.45*	<0.0001	0.0212

B. Test for identical sampling scheme mean:

<u> Horizon</u>	F	P	LSD
A	0.94	0.406	0.029
В	2.22	0.128	0.035

* F is significant at 5% level.

Similar tests as for θ_s^* were done for K_s^* . However, no replicate K_s^* -values were available. Since it has already been shown that K_s^* is lognormally distributed (Table 8), a log transformation was first performed to stabilize the variance. The test results are shown in Table 14. Only the means of the log for horizon A of sampling scheme 3 differed significantly from the means of the B and C-horizon. For all practical purposes we may therefore consider all available K_s^* -data as being one population of which the log transformed mean and standard deviation are 1.730 and 0.5608, respectively.

Once it has been decided that the variable in question follows a normal distribution, one can apply traditional Fisher statistics (Snedecor and Cochran, 1980) to determine the minimum sample size required at a chosen level of probability. In doing so, it can be shown that to estimate the mean θ_s^* of the A-horizon with a tolerated error of 0.01 cm³ cm⁻³, you will need 48 samples at the 95% confidence level. Similarly, if one tolerates a deviation of 10 or 50 cm day⁻¹ in the estimated mean of K_s , one would need 1450 and 58 samples, respectively. However, such a method is unsuited when the distribution of the population being sampled is nonnormal or of a unknown form. An alternative may be to use bootstrapping (Dane et al. 1986), a computer intensive method which has been developed recently.

Table 14. F, P and LSD values to test whether horizon and sampling scheme means of log $K_{\bf S}^{\star}$ are identical.

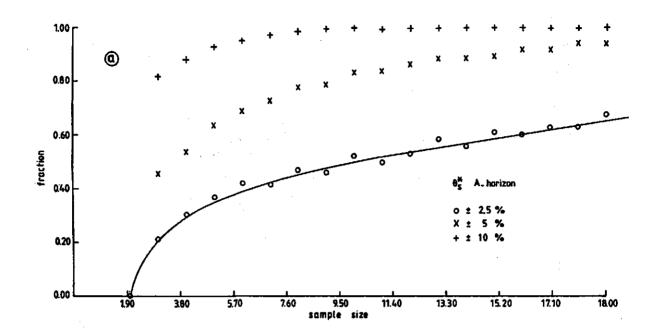
<u>scheme</u>	F	P	LSD	
2 (2 horizons)	0.45	0.5132	0.767	
3 (3 horizons)	5.18*	0.0207	0.489	
2+3 (2 horizons)	0.92	0.4067	0.539	
. Test for identica	al sampling		I CD	
		scheme mean:	LSD	
. Test for identica	al sampling		<u>LSD</u> 0.443	
. Test for identica	al sampling F 11.16*	P		

4. Bootstrapping

The following procedure explains such an application of the bootstrap technique. Bootstrap replicates of size $B=2,\,3,\,\ldots,\,N$ (N is population size) were generated 800 times, and the mean for each replicate calculated. The B random samples are drawn with replacement from the N available observations.

For each value of B, the fraction of the 800 replicates having means within a given percentage of the mean for the N observations is calculated and plotted against the value of B. The intersection point of a curve through the generated points with the horizontal, of which the ordinate is determined by the confidence level, determines the minimum sample size required. Examples for θ_S^* of the A-horizon and K_S^* are shown in Fig. 13 for maximum errors of estimate of 2.5, 5 and 10%.

The results show that the fraction of sample means within the error limits increases with sample size and eventually reaches a plateau beyond little or no additional information is gained. Also a reduction in error limit requires a larger number of observations to estimate the population mean with the same confidence interval. With respect to Fig. 13a, a tolerated error in the mean θ_S^* of 0.01 cm³cm⁻³ corresponds to a maximum error of estimate of 2.5% (open circles in Fig. 13a). Hence, one would need at least an additional 10-15 samples to achieve the required minimum sample size with a confidence level of 95%. A tolerated error in the mean K_S^* of ca. 10 cm day⁻¹ corresponds to a maximum error of estimate of 10% ('+' in Fig. 13b). The 31 samples that were available (A + B horizon) are by far not enough to obtain a reasonable estimate of the population mean of K_S^* .



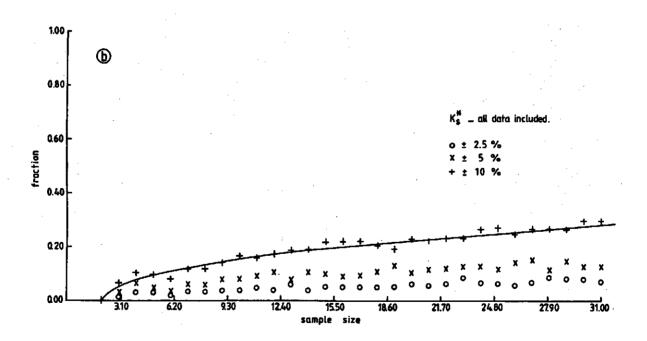


Figure 13. Fraction of samples within the indicated percentage of the maximum error of estimate as a function of bootstrap sample size for θ_s^* A-horizon (a) and K_s^* (b).

5. Spatial dependency

So far it was assumed in the statistical analysis that the soil properties measured make independent samples. However, it is intuitively felt that a soil at near places tends to be similar, whereas that between two distant places is not. An observation therefor carries some information from its neighborhood. Spatial dependence in a soil property can be expressed in terms of a semivariogram, defined as half the expected squared difference between values of places x and x + h. In the theory of regionalized variables (Journel and Huybregts, 1978) the semivariogram is used to predict values of the soil property at nonsampled places or over small areas within a region, by kriging. Fig. 14 shows the semivariograms for $\theta_{\rm S}^{\star}$ of the A and B horizon. Note that the lag distance between locations h is on a log scale. The lag distances for which the semivariance was calculated increases with a larger distance between the sampled points. The distance before the semivariance reaches a plateau value (sill), the range, is a measure for the distance between points where the soil property is spatially dependent. The range of θ_s^* for both horizons appears to be in the neighborhood of 10 m. There is a larger increase in semivariance for the B-horizon than for the A-horizon before the sill is reached, indicating that the saturated water content values in the B-horizon are more spatially dependent.

Semivariograms for $\log K_S^*$ are shown in Fig. 15. Again it appears that the B-horizon has a larger spatial dependence and that the range for both horizons is again ca. 10 m. McBratney and Webster (1983) showed that if spatial dependence is present, the required sampling effort to predict the value of a property at a specific location will be less, than would have been judged necessary using the classical approach.

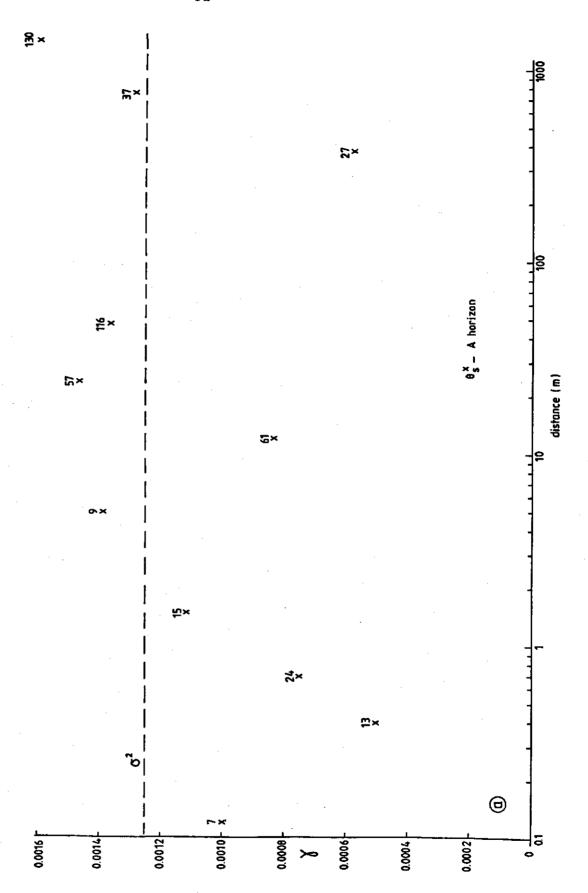


Figure 14a. Semi-variogram of θ_s^* for A horizon. Number near symbols indicate number of paired points to calculate semi-variance.

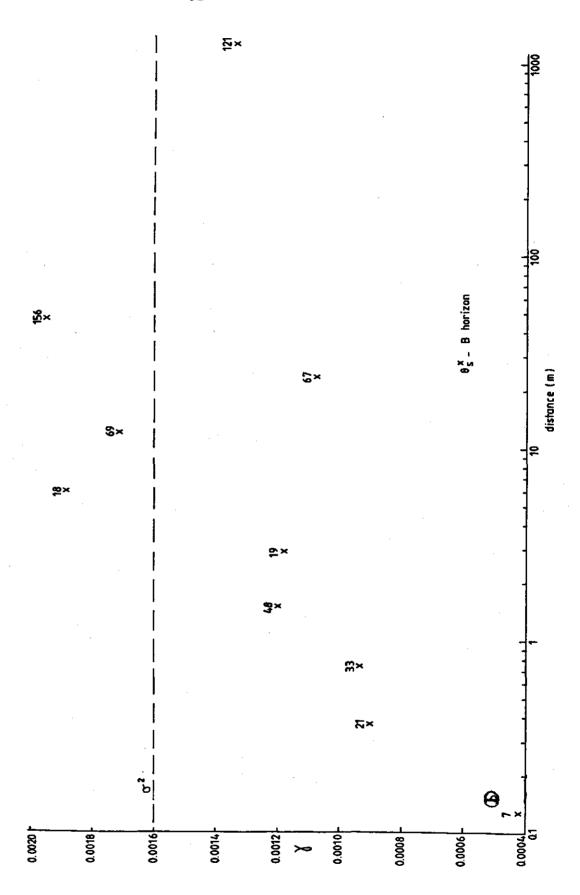


Figure 14b. Semi-variogram of θ_s^* for B-horizon.

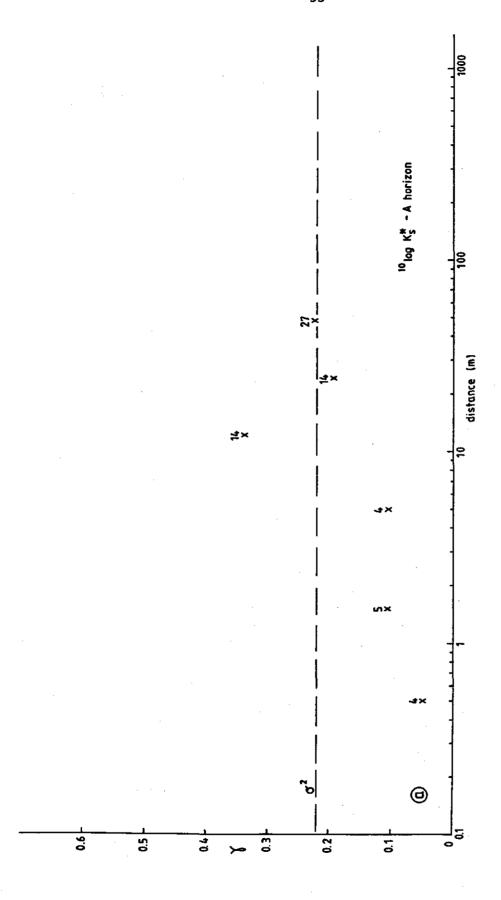


Figure 15a. Semi-variogram of $\log K_s^*$ for A-horizon.

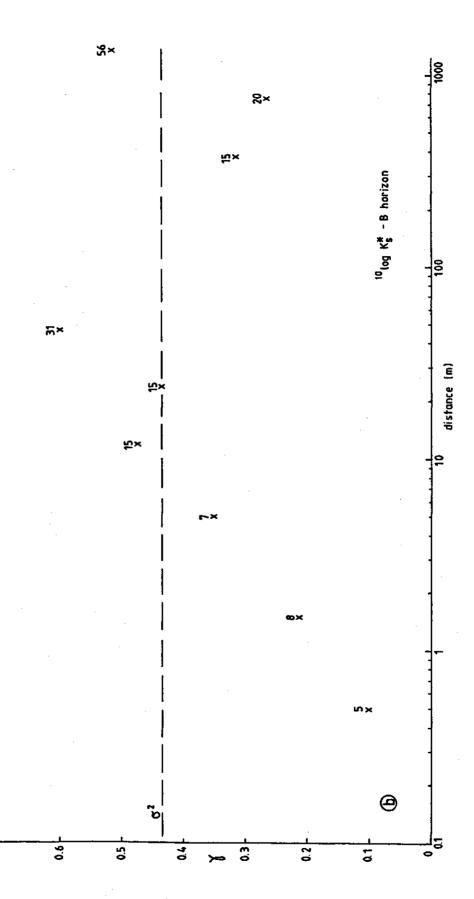


Figure 15b. Semi-variogram of log K_S^* for B-horizon.

6. Scaling

Prediction of water movement in spatial variable soils requires knowledge of the spatial variation of the soil hydraulic properties. A measure of such variation can be obtained by scaling, in particular, the scaling of soil water characteristic curves and hydraulic conductivity data. The theory of scaling is based on the similar media concept, introduced by Miller and Miller (1956). Similar media differ only in the scale of their internal microscopic geometries and have therefore equal porosities and equivalent particle and pore-size distributions. The purpose of scaling is to simplify the description of statistical variation of soil hydraulic properties. By this simplification, the pattern of spatial variability is described by a set of scale factors a, of which each a1 relates the soil hydraulic properties at each location, to a representative mean. Spatial variability is then characterized by the distribution of scale factors. Warrick et al. (1977) extended the application of scaling by estimating scale factors relative to the degree of saturation (s), with the result that the assumption of identical porosities can be eliminated. However, scaling should be restricted to soil locations having some reasonable morphological similarity.

Peck et al. (1977) defined a scaling parameter a₁ as being the ratio of the microscopic characteristic length of a soil and the characteristic length of a reference soil, or

$$a_1 = \lambda_1/\bar{\lambda},$$

where $l=1, \ldots, L$ locations. As a result of scaling one can relate the soil water characteristic and hydraulic conductivity function at any location 1 to an average h_m and K_m , $(a_1=1)$ such that

$$h_1 = h_m/a_1 \tag{4}$$

$$K_1 = a_1^2 K_m,$$
 [5]

According to Eq. [4] and [5], the soil water characteristic and hydraulic conductivity curves of similar soils can be reduced to two single curves,

(scaled mean curves) by means of scaling the soil water pressure head and hydraulic conductivity at each degree of saturation s. Validity of the similar media concept requires that the pressure head (a_1-h) and conductivity scale factors (a_1-K) are equal for each location 1.

Hopmans (1987) investigated various scaling methods as used to obtain scaled mean hydraulic curves. Of these methods, the one introduced by Warrick et al. (1977) were found to be applicable to the Hupsel data. Before scaling, the measured water retention data were fitted by the van Genuchten model (Eq. [1] and [2]). When pressure head and conductivity data of all horizons and sampling schemes combined were each scaled independently, a correlation coefficient of 0.87 was found between a1-h and a1-K. In addition, both a1-h and a1-K were found to be lognormally distributed. Therefore, statistical analysis of the scale factors will only focus on a1-h. It will be assumed that a1-h can be used to describe the variability of the conductivity function, according to Eq. [5].

Since replicate water retention curves were determined for sampling schemes 2 and 3, it was first investigated whether the scale factor values between locations for a given sampling scheme and horizon were significantly different. Water retention curves for each scheme and horizon were scaled independently and analysis of variance was used to test the significance of log-transformed scale factor values between locations. The test results are shown in Table 15. In only one case (scheme 3, C-horizon) a significant difference between locations was found.

Analysis of variance was also used to test whether the mean of the log-transformed scale factor values were identical for the different horizons. All available water retention curves were used in the scaling, however, testing was done for each sampling scheme separately. Table 16 shows that no significant differences were found between the horizons of sampling scheme 3. Since the LSD-value was larger than the difference between the mean scale values of horizon B and C, these two horizons were combined and the analysis of variance repeated. There was now no significant difference between the A-horizon and underlying soil for all three sampling schemes.

a . l	6 4		Horizon	
Scheme	Statistics	Α	В	C
2	F	3.88	0.28	
	P	0.0501	0.929	
	LSD	0.4618	0.682	
3	F	2.92	1.05	7.38*
	P	0.117	0.440	0.0039
	LSD	0.245	0.391	0.264

Table 15. F, P and LSD values to test whether location means of \$^{10}\log\$ (a) are identical

Scheme				log mean scale factor values		
	F	P	LSD	A	В	С
1	16.29*	0.0020	0.2467	0.1494	-0.3038	-
2	7.83*	0.0096	0.1327	-0.4567	-0.2761	-
3	3.09	0.0563	0.154	-0.0454	0.1469	0.09816
3@	5.79*	0.0207	0.141	-0.454	0.1225	-

[@] Combination of B and C horizon

Table 16. Statistics, to test for identical log-transformed means of scaling factor values between horizons.

To investigate differences between sampling schemes, the A and underlying horizont (B, and BC horizont for sampling scheme 3) were each scaled independently. Analysis of variance showed that the mean of the log-transformed scaling factors between all three schemes were significantly different for both horizons (Table 17). In general, the standard deviation in log(a) decreased with a smaller sampled area. Only the variance of scheme 3 for the the BC horizont did not follow this general behaviour. Since the samples of schemes 2 and 3 were part of the same soil type, one would expect statistically insignificant differences between schemes 2 and 3, and certainly a decrease in variance when comparing scheme 3 with 2.

Nevertheless, all three sampling schemes were combined and a mean and standard deviation of log(a) was calculated for each horizon. These statistics are listed in Table 18, which also shows that both distributions follow indeed a lognormal distribution (KS<0.895).

When the distribution of scale factor values is used to generate scale factor values (as in Monte Carlo analysis), it is important to notice that the scaled mean hydraulic functions are described with s (degree of saturation) as the independent variable. Also $\theta_{\rm S}$ has a known distribution (page 46). So if there exists a correlation between $\theta_{\rm S}$ and scale factor value, the two variables can not be generated independently of each other. R²-values between $\theta_{\rm S}$ and log(a) were calculated to be 0.0017 and 0.0713, respectively, for the A and B horizon. I.e. $\theta_{\rm S}$ can be drawn independently of log(a).

Since the unscaled water retention data were fitted by the van Genuchten model, it was further investigated if scale factor values could be predicted by the parameters α and n (Eq. [1]). As might be expected, regression resulted in high correlation coefficient values. Table 19 also shows the regression coefficient for each horizon separately, while calculated and predicted scale factor values are compared in Fig. 16. It would further be of interest if scale factors could be predicted from textural data, as was shown by Vauchaud et al. (1986). However, textural analysis was done for only a limited number of soils in Hupsel. In addition textural differences between soils where presumably too small to find any significant correlation.

<u>A-horizon</u>		F	Р	LSD
		15.93*	<0.0001	0.239
-	scheme	<u>и</u> log а	σlog a-	
	1 (7)	-0.1434	0.2993	
	2 (14)	-0.3730	0.2364	
	3 (11)	-0.2302	0.1148	
B(C)-horizo	<u>on</u>	F	Р	LSD
		17.43*	<0.0001	0.179
_	scheme	<u>μ</u> log a		
	1 (6)	0.0472	0.1971	
	2 (14)	-0.3404	0.1530	
	3 (32)	0.0253	0.2174	

Table 17. Statistics to test for identical log-tranformed means of scaling factor values between sampling schemes.

hori	zon	$^{\mu}$ log a	σ log a	KS
A	(32)	-0.1154	0.3430	0.844*
B(C)	(52)	-0.0706	0.2567	0.853*

Table 18. Mean and standard deviation of 10log a of A and B(C)-horizon, when all 3 sampling schemes were combined.

	****	Regres	sion coei	ficient		Coefficient of
<u>A-horizon</u>	b o	ь 1	b 2	b ₃	b ₄	determination R ²
(32)	-5.2638	3.6872	30.3091			0.844
	-0.9265	-2.6866	61.0696	2.1472	-523.091	0.900
BC-horizon	-1.1874	0.9817	8.8800			0.8826
(52)	-2.5713	1.8565	35.7843	-0.1780	-271.346	0.945

Table 19. Regression and correlation coefficients for prediction of scale factor values from α and n; a=b_0+b_1n+b_2 α (+b_3n^2+b_4 α ^2).

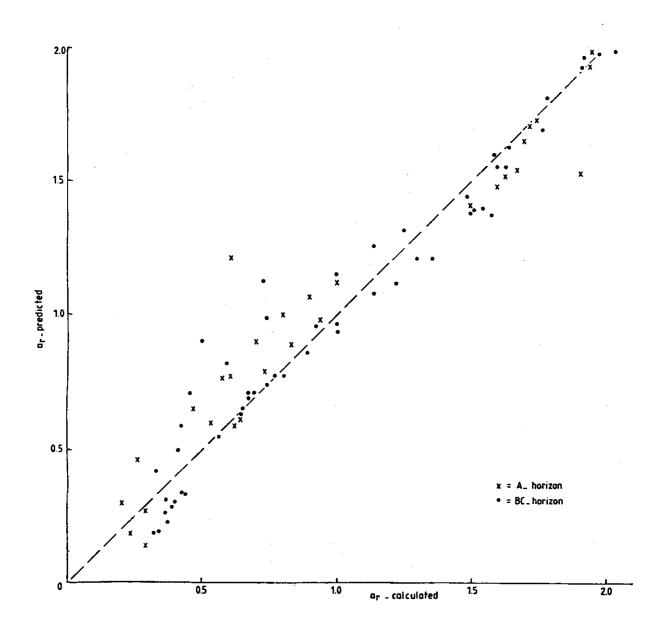


Figure 16. Plot of calculated versus predicted scale factor values for 3 sampling schemes combined.

In addition to the water retention curves, also all available conductivity data were scaled for the two horizons separately. The scaled mean water retention curves and hydraulic conductivity functions for both horizons are shown in Fig. 17 and 18, respectively. Van Genuchten's modified curve fitting procedure (RETC) was used to fit both the soil water characteristic and hydraulic conductivity function simultaneously. The fitted parameters to describe the hydraulic functions (Eq. [1] and [2]) are listed in Table 20.

If the soils at the sampled locations of the Hupsel watershed were perfect similar media, then the set of scale factor values calculated from water retention data (a-h) would have been identical to those calculated from conductivity data (a-K). Hence, a plot of a-h versus a-K values should fall along the 1:1-line.

It must be remembered, however, that although the sample replicates were from the same horizon, $\theta(h)$ and $K(\theta)$ were measured from different samples. Given the variation that already existed between the replicates, it should come as no surprise that the a_h - a_K plot (Fig. 19) exhibits a rather wide band. Better agreement between the two scale factor values would have been obtained if the replicate hydraulic properties were combined before scaling (Hopmans, 1987).

Inclusion of spatial dependency of the soil hydraulic properties in 2-dimensional water flow simulation requires knowledge of the spatial structure of the relevant properties in the 2-dimensional plane. Since it is proposed to express the variability of both the water retention curve and the hydraulic conductivity function with the single scaling parameter a₁-h, semivariograms of the scaling factor for both the A and B horizon are necessary. The semi-variograms of both horizons are displayed in Fig. 20. Similarly to the semi-variograms of θ_S^* and $\log K_S^*$ (Fig. 14 and 15), spatial structure is apparent up till a between point distance of ca. 10 meter. Values of the semi-variance and overall variance for Θ_S^* , $\log K_S^*$, and scale factor a are listed in Table 21. The values in this table can be used to derive the appropriate semi-variogram, which is necessary to generate 2-dimensional fields of the parameter in question using for example the nearest neighbor autoregressive model (Smith and Freeze, 1979). To check for

dependence between the calculated a-values in the vertical direction, the correlation coefficient (R) between the scale factor values of the A-horizon and B-horizon for all sampling schemes combined was calculated to be 0.140. I.e., scale factors values between the A and B horizont were virtually independent.

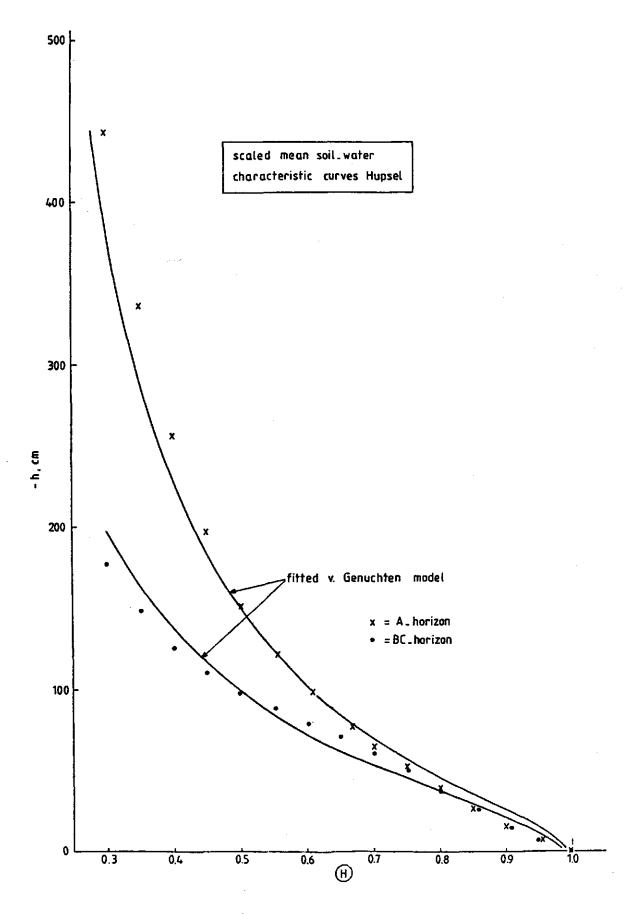


Figure 17. Scaled mean water retention curves Hupsel of A and BC-horizon.

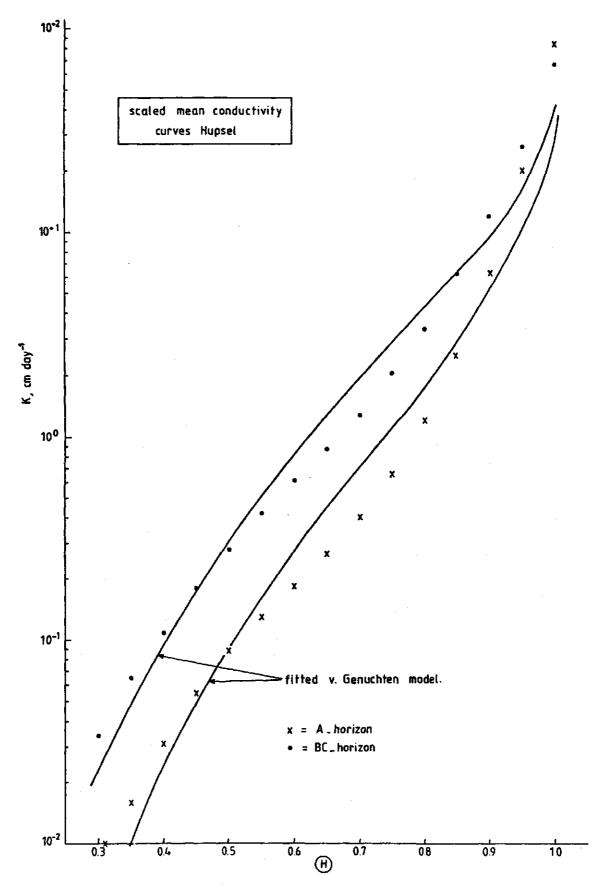


Figure 18. Scaled mean hydraulic conductivity curves Hupsel of A and BC-horizon.

Parameter	Horizon			
	A	ВС		
θ _r	0.00	0.00		
$\theta_{\mathtt{S}}$	0.4024	0.3195		
α	0.01924	0.02043		
n	1.5931	1.8187		
K_s (cm day ⁻¹)	33.7	40.55		

Table 20. Parameters of van Genuchten model to describe scaled mean hydraulic functions for A and BC-horizon.

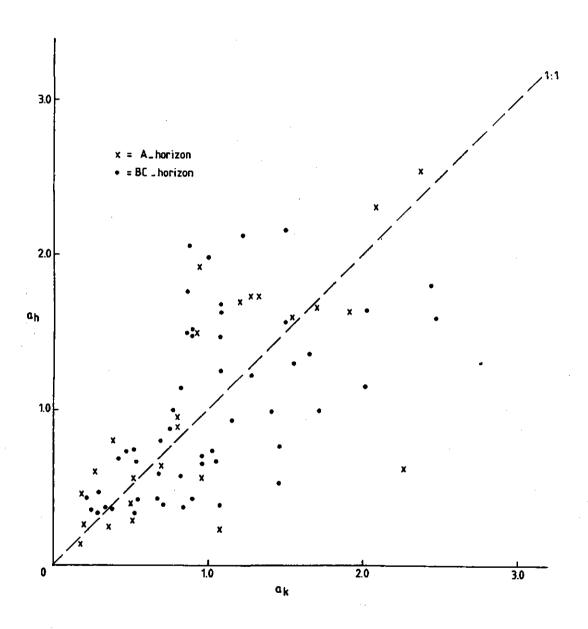


Figure 19. Comparison of scale factor values, as calculated from water retention (a-h) and conductivity (a-K) data.

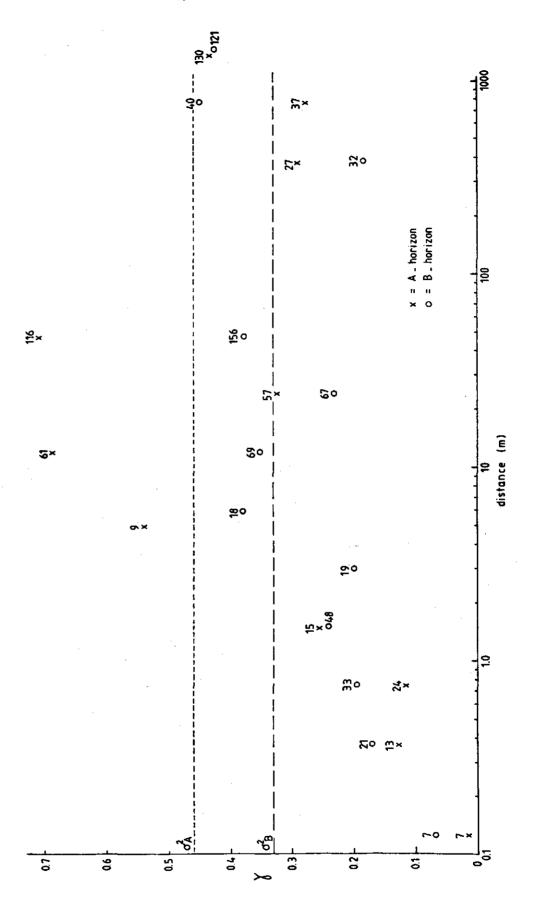


Figure 20. Semi-variograms of scale factor values of A and BC-horizon.

Variable	Horizon	Variance	Semi-variance at distance					
			0.125	0.375	0.75	1.5	3	
θ*	A	.125*10-2	.100*10-2	.497*10-3	.757*10-3	.107*10-2	-	
	В	.160*10 ⁻²	.311*10-3	.921*10 ⁻³	.933*10 ⁻³	.120*10-2	.120*10-2	
log Ks	A	0.221	-	-	0.0481	0.107	-	
	В	0.432	-	-	0.103	0.216	-	
a	A	0.461	0.0151	0.127	0.114	0.257	•	
	В	0.329	0.075	0.172	0.196	0.239	0.200	

Semi-variance at distance (m)								
Variable	Horizon	5	6	12	24	48	384	768
θ_{S}	A	.139*10-2	_	.835*10-3	.147*10-2	.136*10-2	.562*10-2	.129*10-2
	В	-	.189*10 ⁻²	.171*10 ⁻²	.108*10 ⁻²	.197*10 ⁻²	.480*10 ⁻²	.805*10 ⁻³
log K _s	A	0.103	-	0.349	0.188	0.228	-	-
	В	0,326	-	0.471	0.433	0.591	0.313	0.259
a	A	0.542	-	0.690	0.322	0.709	0.294	0.272
	В	-	0.380	0.350	0.229	0.372	0.175	0.452

Table 21. Semi variance values of θ_s^* , log K_s^* and a for A and B horizon.

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APPEND	<u>ΙΧ</u>				
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.092	383				
0.102	272				
0.130	554				
0.144	634				
0.146	109				
0.164	106				
0.182 .231	286 169				
.231	87				
,232	144				
.265	59				
.275	48				
.276	77				
. 284	43				
.321	14				
.357	13				
.385	5				
. 396	5				
1121	242778.8	453068.2	35.0	0 4007	
11	29	0.0200	1.7501	0.4207	25.04
.110	321				
.148 .173	134 134				
.230	77				
.230	105				
.272	105				
.309	41				
.318	37				
.391	41				
. 398	5				
.438	5				
.0032	332	0.110		SORP	
.0071	237	0.122			
.0115	148	0.148			
.0121	112	0.185			
.0387 .0738	125 121	0.17 0.175			
.0305	109	0.189			
.0501	100	0.2			
.0529	75	0.275			
.1026	124	0.17			
.1957	112	0.185			
.1724	104	0.19			
. 2710	81	0.255			
.192	76	0.275			
.5482	95	0.222			
.7833	95	0.222			
1.182 .5257	89 77	0.24 0.273			
. 5329	62	0.273			
2.159	62	0.29			
4.064	48	0.32			
5.576	29	0.365			
9.479	27	0.37			
10.66	18	0.391		CRUST	
14.75	12	0.408			
24.14	11	0.408			
31.4	9	0.413			
79.85	2 1	0.43			
134.7 1131	242778.8	0.438 453068.2	110.0		
10	5	0.0107	4.4548	.3125	5.30
10	,	0.0107	7.7570		2.00

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    .169
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                 89
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                 80
    .268
                 69
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    .314
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    .33
                         0.10
                                            SORP
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                                            CRUST
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                         0.329
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    .283
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    .285
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59.74
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                                 1.5800
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                 5
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	2.31				

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   .347
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                                            SORP
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  .1900
                180
                 43
                          .271
  .2885
                          .330
                                            CRUST
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10.47
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2322 6	242462.0	0.0344	1.3317	0.3452	695.0
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55.7200 18.5000	8 30	0.379 0.301					
6.1800	34	0.301					
0.0600	46	0.261					
0.3100	42	0.270					
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0.6007	90	0.197	0.00700				•
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0.0146	265	0.117	0.00076				
3521	242509.4	453081.9	59.0	0 2000	10.0	0.0010	0 0100
21	59 3	0.0169	2.4484	0.3080	18.8	0.0219	2.2183
0.309 0.289	10						
0.267	32						
0.211	63						
0.125	100						
0.061	148						
0.051	331						
0.338	3						
0.320	10						
0.291	32						
0.209	63						
0.126	100						
0.068	148						
0.050 0.291	331 3						
0.291	10						
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151.9900	0	0.308		CRUST			

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3621 14	242509.4 45	453081.3	59.0 2.3654	0.2910	18 3	0.0331	1,8683
0.306	3	0.0139	2.3034	0.2710	10.5	0.0331	1,0005
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6 0.357 0.341 0.248 0.096	0 3 10 32 100	0.0378	1.8423	0.3623	323.4
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0.313 0.254 0.168 0.102 0.041	10 32 100 159 501		•		
3135 7 0.348 0.344 0.347	242507.1 0 1 3	453082.0 0.0226	90.0 1.6309	0.3469	31.6
0.269 0.199 0.164 0.054 3138	32 100 159 501 242507.4	453081.7	90.0		
7 0.303 0.302 0.298 0.25	0 1 3 10 32	0.0074	2.2002	0.2898	31.6
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0.306 0.305 0.292 0.24 0.105	1 3 10 32 100				
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0.23	63				
0.172	100				
0.144	148				
0.081	331				
3412	242508.8	453081.4	0.35		
7	0	0.0237	1.7821	0.4075	75.5
0.400	3				
0.394	10				
0.34	32				
0.28	63				
0.145	100				
0.134	148				
0.117	331				
3418	242508.6	453081.1	35.0		
7				0 4204	7E E
_	0	0.0231	1.6706	0.4304	75.5
0.423	3				
0.418	10				
0.363	32				
0.302	63				
0.2	100				
0.162	148				
0.134	331		44.4		
3422	242508.8	453081.4	60.0		
7	0	0.0185	2.1928	0.3529	62.2
0.351	3				
0.346	10				
0.306	32				
0.241	63				
0.124	100				
0.091	148				
0.071	331				
3425	242508.4	453081.4	60.0		
7	0	0.0130	2.3811	0.2735	62.2
0.28	3	0.0130	2.3022		
0.268	10				
0.254	32				
0.205	63				
0.157	100		•		
0.091	148				
0.039	331		•		
3428	242508.6	453081.1	60.0		
				0 2007	60.0
7	0	0.0189	1.6179	0.2907	62.2
0.29	3				
0.281	10				
0.258	32				
0.211	63				
0.174	100				
	148				
0.138					
0.097	331		 -		
3432	242508.8	453081.4	90.0		
7	0	0.0124	2.2924	0.2957	45.5
0.298	3				
0.290	10				
0.277	32				
0.237	63				
0.167	100				
0.113	148				
0.055	331				
0.000					

3435	_	453081.4	90.0		
7 0.296	0 3	0.0137	1.5393	0.2989	45.5
0.292	10				
0.28	32				
0.25	63				
0.206	100				
0.18 0.133	148 331				
3438	242508.6	453081.1	90.0		
7	0	0.0170	1.5006	0.2939	45.5
0.298	3				
0.28 0.265	10				
0.233	32 63				
0.195	100				
0.172	148				
0.121	331				
3512 7	242509.6	453082.0	28.0	A 2700	010 2
0.363	0 3	0.0358	1.5232	0.3700	212.3
0.347	10				
0.298	32				
0.215	63				
0.175 0.132	100 148				
0.132	331				
3515	242509.2	453082.0	28.0		
7	0	0.0603	1.4667	0.4400	212.3
0.439	3				
0.373 0.308	10 32				
0.231	63				
0.184	100			- •	
0.134	148				
0.122 3518	331 242509.4	453081.7	28.0		
7	242303.4	0.0299	1.5722	0.3930	212.3
0.372	10	3.72.7	_,,_,		
0.388	3				
0.324	32				
0.241 0.193	63 100				
0.146	148				
0.124	331				
3522	242509.6	453082.0	59.0		
7	0 3	0.0158	2.4824	0.2999	18.8
0.309 0.289	10				
0.267	32				
0.211	63				
0.125	100				
0.061	148				
0.051 3525	331 242509.2	453082.0	59.0		
7	0	0.0181	2.3593	0.3319	18.8
0.338	3				
0.32	10				
0.291 0.209	32 63				
0.209	100				
0.068	148				
0.050	331				
3528	242509.4	453081.7	59.0		

7	0	0.0160	2.5981	0.2862	18.8
0.291	3	0,0100	2.3901	0.2002	10.0
0.28	10				
0.26	32				
0.184	63				
0.127	100				
0.052	148				
0.031	331				
3532	242509.6	453082.0	90.0		
7	0	0.0158	2.7589	0.2687	8.1
0.274	3				
0.263	10				
0.243	32				
0.182	63				
0.098	100				
0.047	148				
0.037 3535	331 242509.2	453082.0	90.0		
3333 7	242309.2	0.0147	3.2663	0.2751	8.1
0.283	3	0.0147	3.2003	0.2/31	0.1
0.271	10				
0.256	32				
0.182	63				
0.109	100				
0.028	148				
0.017	331				
3538	242509.4	453081.7	90.0		
7	0	0.0145	3.1589	0.2767	8.1
0.287	3				
0.269	10				
0.256	32				
0.195	63				
0.1	100				
0.041 0.032	148 331				
3612	242509.6	453081.4	28.0		
7	0	0.0481	1.4475	0.3700	190.6
0.368	3	3,3,42	2,41,5	0.0,00	2,0.0
0.328	10				
0.282	32				
0.212	63				
0.173	100				
0.134	148				
0.123	331				
3618	242509.4	453081.1	28.0	0.000	100 (
7	0	0.0341	1.6885	0.3639	190.6
0.361 0.339	3 10				
0.279	32				
0.19	63				
0.147	100				
0.099	148				
0.09	331				
3615	242509.2	453081.4	28.0		
7	0	0.0314	1.5683	0.4090	190.6
0.409	3				
0.382	10				
0.334	32			Y	
0.249	63				
0.195	100				
0.143	148				
0.131 3625	331 242509.2	453081.4	59.0		
3623 7	242309.2	0.0178	2.3751	0.2998	18.3
,	U	0.0178	2.J/JL	0.2330	10.3